

西南交通大学 2002 年硕士研究生招生考试

材料力学

试题

考试时间：2002 年 1 月

考生请注意：

1. 本试题共 7 题，共 12 页，考生请认真检查；
2. 答题时，直接将答题内容写在试题卷上；
3. 本试题不得拆开，拆开后遗失后果自负。

题号	一	二	三	四	五	六	七	八	九	总分
得分										
签字										

一、图示结构 C 结点与滑块铰接，不计滑块与滑槽间摩擦力，滑块只可沿滑槽上下自由移动，AB 与 BC 两杆面积相同且均为钢制，面积 $A=100\text{mm}^2$ ，材料拉压弹性模量 $E=2.0\times 10^5\text{MPa}$ ，线膨胀系数 $\alpha=12\times 10^{-6}(1/^\circ\text{C})$ 求当 BC 杆升温 50°C ，而 AC 杆温度不变时 C 处位移值。（15 分）

AC 杆内力 N_2 BC 杆内力 N_1

$$N_2 \cos 30^\circ = N_1$$

若无 AC 杆，则温度升高 AC 杆伸长到 C_1 ，无温度变化

而 AC 杆伸长，沿 C 点圆筒 Δl_2

$$\Delta l_1 = \frac{\Delta l_2 EA}{E}$$

$$N_1 = \frac{(\Delta l_1 - \Delta l_2) \cos 30^\circ EA}{l_2}$$

AC 杆内力

$$l_2 \cos 30^\circ = l_1$$

$$\frac{(\Delta l_1 - \Delta l_2) \cos 30^\circ EA}{l_2} \cos 30^\circ = \frac{\Delta l_2 EA}{l_2 \cos 30^\circ}$$

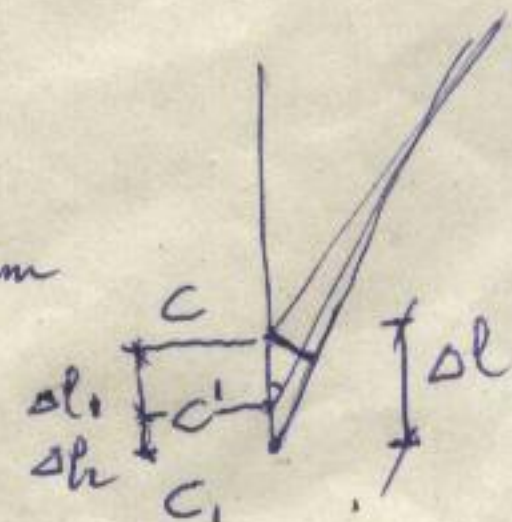
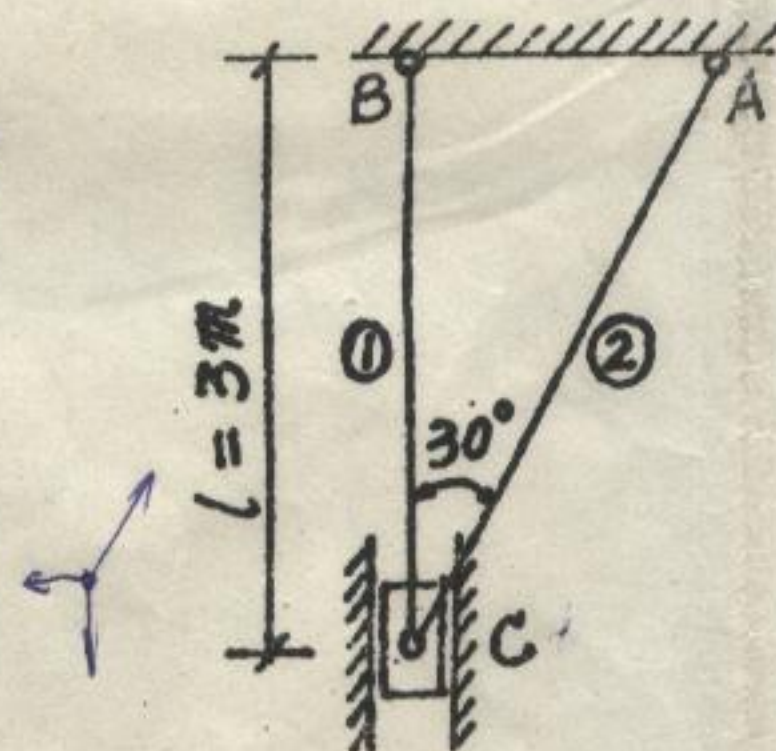
$$(\Delta l_1 - \Delta l_2) \cos^2 30^\circ = \Delta l_2$$

$$\Delta l_1 = \alpha \Delta t l_1 = 1800 \times 10^{-6} \text{ m} = 1.8 \text{ mm}$$

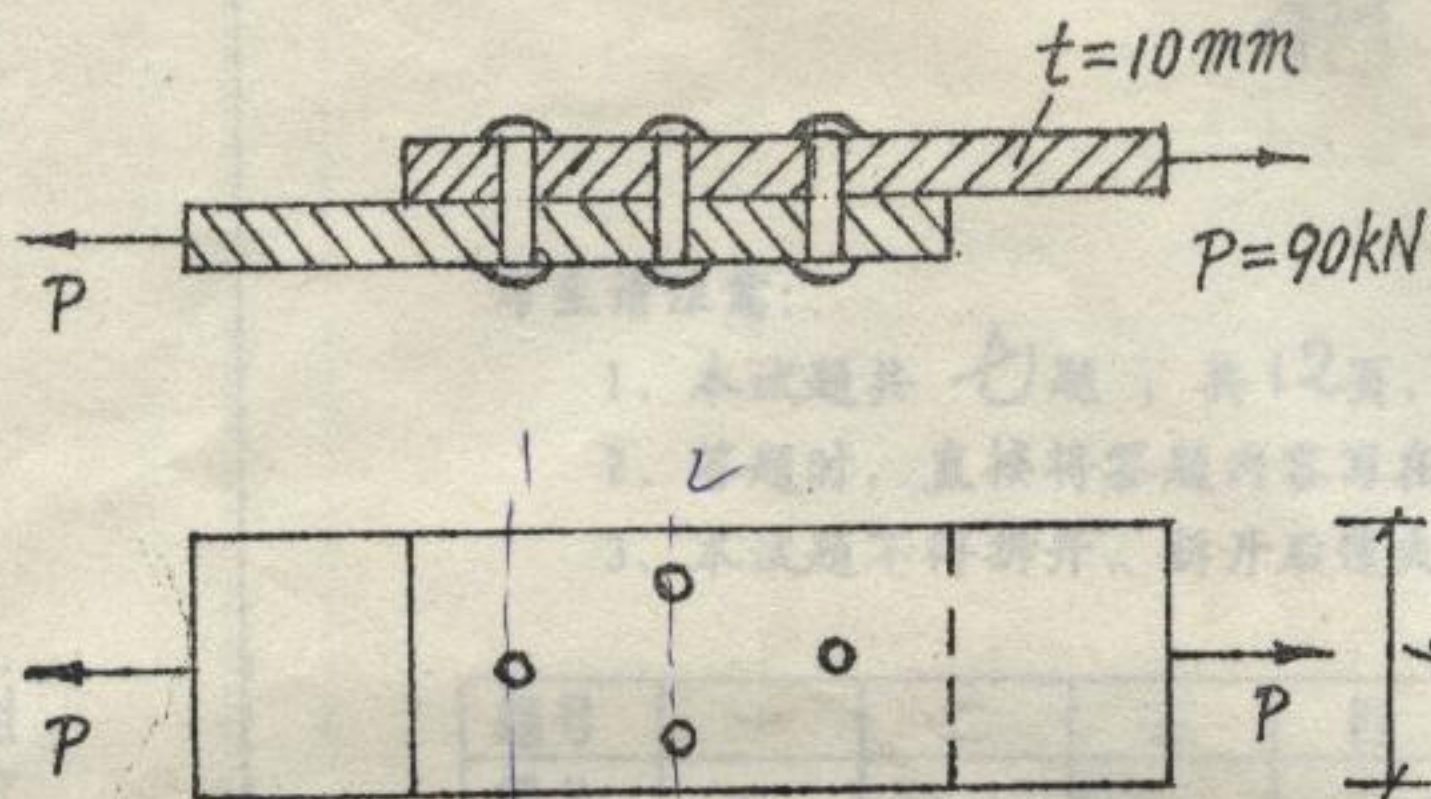
$$1.8 - \Delta l_2 \frac{3\sqrt{3}}{8} = \Delta l_2$$

$$\Delta l_2 = 0.708 \text{ mm}$$

$$\Delta l_1 = \Delta l_1 - \Delta l_2 = 1.8 - 0.708 = 1.091 \text{ mm}$$



二、一拉杆接头如图所示，板厚 $t=10\text{mm}$ ，已知板与铆钉材料许用剪应力 $[\tau]=120\text{MPa}$ ，许用挤压应力 $[\sigma_c]=340\text{MPa}$ ，许用拉应力 $[\sigma]=160\text{MPa}$ ，试设计铆钉直径 d 及板宽 b 值。（15分）



力通过铆钉的圆心，故铆钉受剪。

铆钉受剪

$$\tau = \frac{Q}{A} = \frac{90 \times 10^3}{\pi d^2/4} \leq [\tau] = 120 \text{ MPa}$$

$\Rightarrow d \geq \sqrt{\frac{90 \times 10^3}{\pi \cdot 120}} = 15.45 \text{ mm}$
 $d \geq 16 \text{ mm}$

铆钉受挤压

$$\sigma_c = \frac{P/4}{td} = \frac{90 \times 10^3}{4 \times 10 d} \leq [\sigma_c] = 340 \text{ MPa}$$

$$d \geq \frac{90 \times 10^3}{4 \times 10 \times 340} = 6.6 \text{ mm}$$

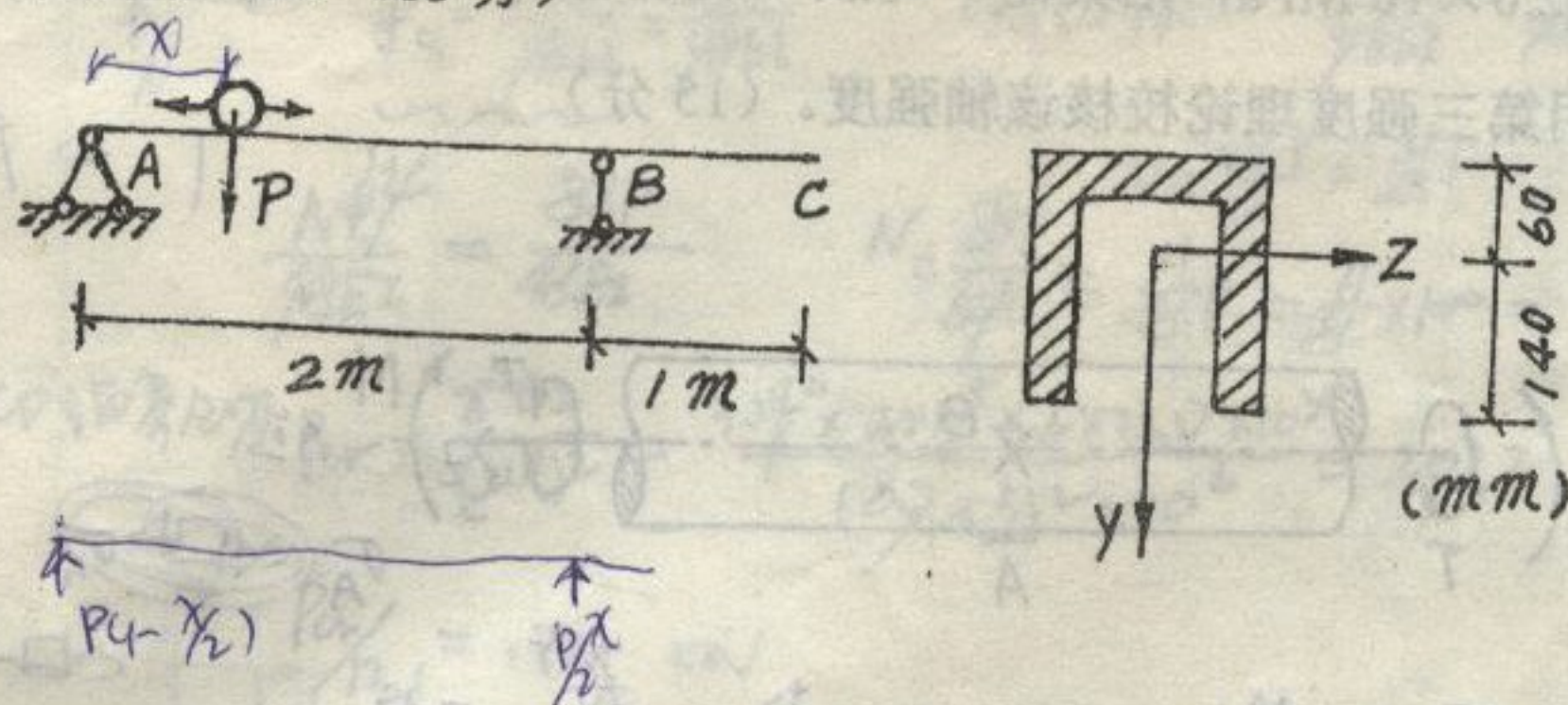
与板受拉

$$\sigma_c = \frac{P}{bt - 2dt} = \frac{90 \times 10^3}{(b - 2d)t} \leq [\sigma] = 160 \text{ MPa}$$

$$d \geq 20 \text{ mm}$$

$$b \geq 100 \text{ mm}$$

四、图示外伸梁上荷载 P 可沿梁水平移动，梁截面为槽形， $I_z = 4.0 \times 10^7 \text{ mm}^4$ ，梁材料许用拉应力 $[\sigma_{\text{拉}}] = 35 \text{ MPa}$ ，许用压应力 $[\sigma_{\text{压}}] = 140 \text{ MPa}$ ，求该梁的容许荷载 P 值。(15 分)



P 在 AB 中点 (1m) 处 $\frac{1}{2}P \uparrow$ $\sigma = \frac{M}{I} y$

$$\sigma_{\text{拉}} = \frac{\frac{P}{2} \times 1000}{4 \times 10^7} \times 140 = 1.75 P \leq [\sigma_{\text{拉}}] = 35 \text{ MPa}$$

$$\sigma_{\text{压}} = \frac{\frac{P}{2} \times 1000}{4 \times 10^7} \times 60 = 0.75 P \leq [\sigma_{\text{压}}] = 140 \text{ MPa}$$

P 在 C 处 B 点 M 最大

$$\sigma_{\text{拉}} = \frac{P \times 1000}{4 \times 10^7} \times 60 = 1.5 P \leq [\sigma_{\text{拉}}] = 35 \text{ MPa}$$

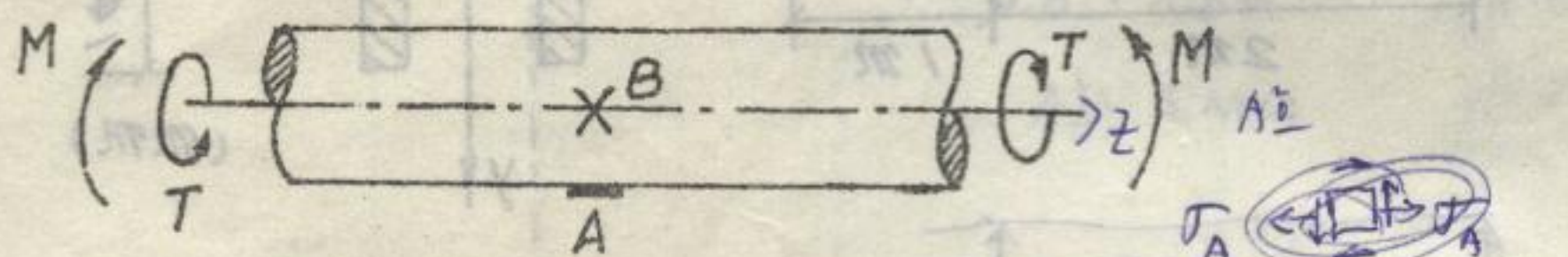
$$\sigma_{\text{压}} = \frac{P \times 1000}{4 \times 10^7} \times 140 = 3.5 P \leq [\sigma_{\text{压}}] = 140 \text{ MPa}$$

$$\begin{cases} P \leq 20 \text{ kN} \\ P \leq 18 \text{ kN} \\ P \leq 23 \text{ kN} \\ P \leq 40 \text{ kN} \end{cases}$$

\therefore 容许荷载 20 kN

五、图示钢制圆轴受弯矩 M 及扭矩 T 联合作用，圆轴直径 $d=18\text{mm}$ ，
现测得圆轴表面最低 A 点处沿轴向线应变 $\varepsilon_0 = 5.0 \times 10^{-4}$ ，测得水平轴切面
上 B 点处 45° 方向上线应变 $\varepsilon_{45^\circ} = 4.2 \times 10^{-4}$ 及 $\varepsilon_{-45^\circ} = -4.2 \times 10^{-4}$ ，已知钢弹
性模量 $E=2.0 \times 10^5 \text{MPa}$ ，泊桑比 $\nu=0.3$ ，许用应力 $[\sigma]=170 \text{MPa}$ ，试求 M 及
 T 值，并用第三强度理论校核该轴强度。(15 分)

$T \Rightarrow$ 左端为负，右端为正



$$\begin{cases} I_z = \frac{\pi d^4}{64} \\ I_y = \frac{\pi d^4}{32} \\ W_z = \frac{\pi d^3}{32} \\ W_y = \frac{\pi d^3}{16} \end{cases}$$

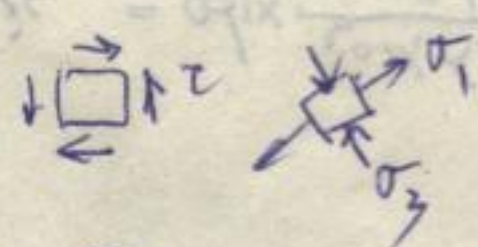
A 点: $\sigma_A = \frac{M}{W_z}$ $W_z = \frac{I_z}{y} = \frac{\frac{\pi d^4}{64}}{\frac{d}{2}} = \frac{\pi d^3}{32}$
 $\varepsilon_0 = \frac{\sigma_A}{E} \Rightarrow \varepsilon_0 E = \frac{M}{W_z}$

$$M = \varepsilon_0 E W_z = 5.0 \times 10^{-4} \times 2.0 \times 10^5 \times \frac{\pi \times 18^3}{32}$$

$$\sigma_1 - \sigma_3 = \varepsilon_0 E = 5.0 \times 10^{-4} \times 2.0 \times 10^5 = 100 \text{MPa} < [\sigma] = 170 \text{MPa}$$

B 点: 在 45° 方向. B 点处 $\sigma_x = 0$ $\tau_y = \frac{Q_s}{bZ}$
 $\tau_y = 0$

在 T 作用下 B 点



$$\tau = \frac{T}{W_p}$$

$$W_p = \frac{\pi d^3}{16}$$

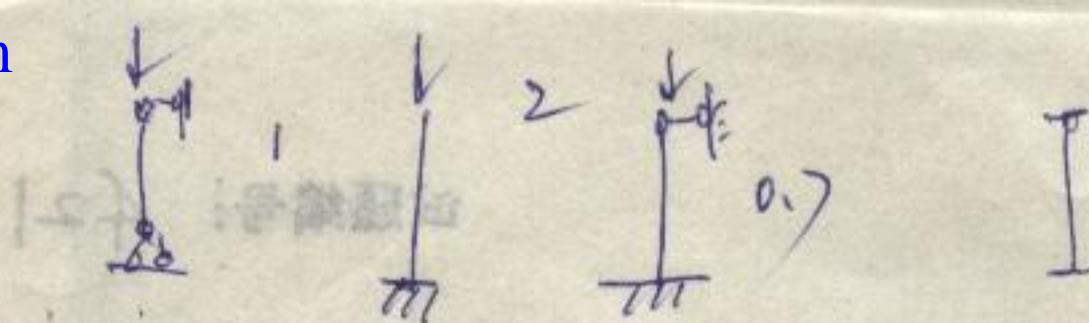
$$\varepsilon_{45^\circ} = \frac{1}{E} (\sigma_1 - \nu \sigma_3)$$

$$E \varepsilon_{45^\circ} = T \left(\frac{1}{W_p} + \frac{\nu}{W_p} \right)$$

$$T = \varepsilon_{45^\circ} E W_p / (1 + \nu) = 4.2 \times 10^{-4} \times 2.0 \times 10^5 \times \frac{\pi \times 18^3}{16 \times (1 + 0.3)} =$$

$$\sigma_1 - \sigma_3 = \frac{2T}{W_p} = \frac{2 \times 4.2 \times 10^{-4} \times 2.0 \times 10^5 \times \frac{\pi \times 18^3}{16}}{2 \times 10^5 \times (1 + 0.3)} = \frac{168}{1.3} = 129.2 \text{MPa} < [\sigma] = 170 \text{MPa}$$

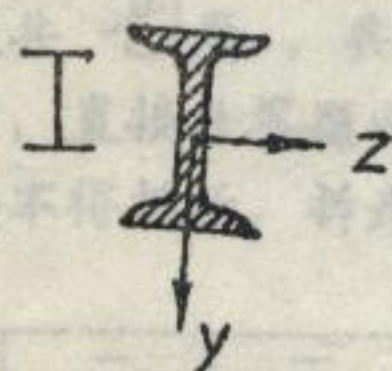
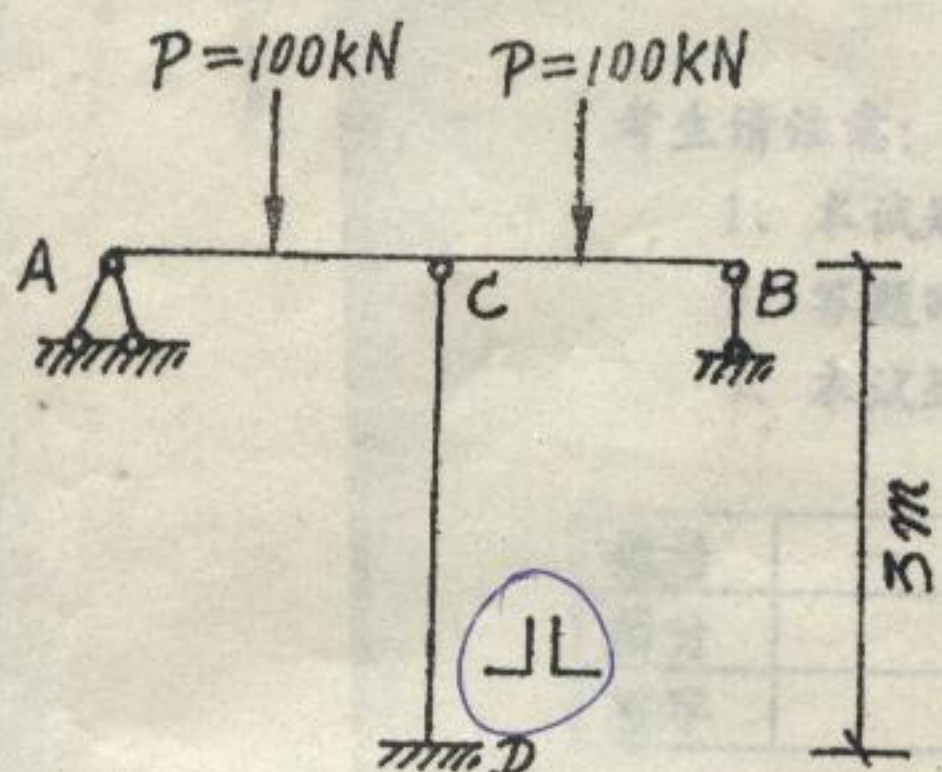
$$\begin{cases} I_p = \frac{\pi d^4}{32} \\ I_{max} = \frac{I_p}{W_p} \end{cases}$$



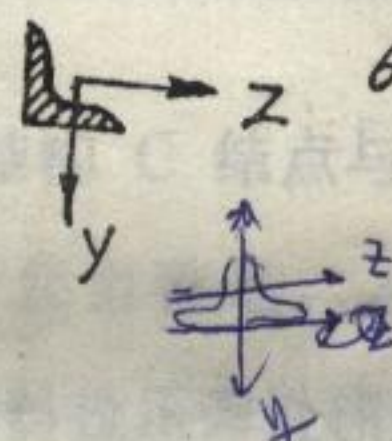
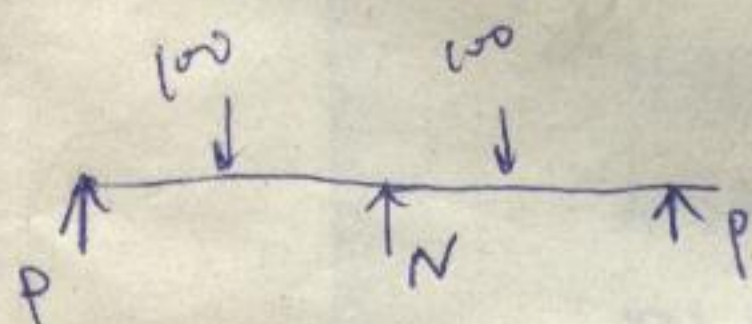
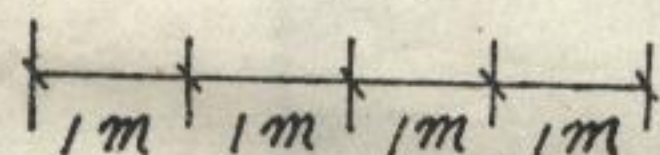
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六、梁 AB 为 16# 工字钢，立柱 CD 为两根 6.3# 等边角钢拼装而成，立柱与梁联接处为铰接，立柱下端为固定端，已知材料弹性模量 $E=2.0 \times 10^5 \text{ MPa}$ ，比例极限 $\sigma_p = 190 \text{ MPa}$ ，取稳定安全系数 $n_w = 1.8$ ，若不计 CD 柱压缩变形量，试校核 CD 立柱稳定性。(15 分)

田修明



16# 工字钢 $I_z = 11.30 \times 10^6 \text{ mm}^4$
 $W_z = 141 \times 10^3 \text{ mm}^3$



6.3# 角钢 $I_y = I_z = 23.17 \times 10^4 \text{ mm}^4$
 $A = 614.3 \text{ mm}^2$
 $i_y = i_z = 19.4 \text{ mm}$

拼装后的角钢 $I_z = 2I_{z1} = 2 \times 23.17 \times 10^4 \text{ mm}^4$
 $A = 2A_1 = 614.3 \times 2 \text{ mm}^2$
 $i_z = \sqrt{\frac{I_z}{A}} = \sqrt{\frac{2I_{z1}}{2A_1}} = i_{z1} = 19.4 \text{ mm}$

立柱 CD 的长细比为 λ

由欧拉公式

$$\lambda = \frac{\mu l}{i_z} = \frac{0.7 \times 3000}{19.4} = 108 > \lambda_p$$

由欧拉公式

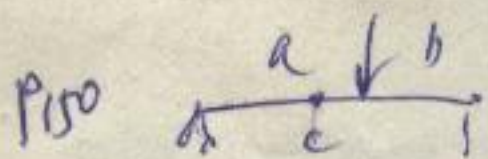
$$\begin{cases} \sigma = \frac{P}{A} \\ P_{cr} = \frac{\pi^2 EI}{(\mu l)^2} \end{cases}$$

$$P_{cr} = \frac{\pi^2 EI}{(\mu l)^2}$$

$$= \frac{\pi^2 E}{\left(\frac{\mu l}{i_z}\right)^2} = \frac{\pi^2 E}{\lambda^2} \leq \sigma_p$$

$$\lambda \geq \sqrt{\frac{\pi^2 E}{\sigma_p}}$$

$$\sqrt{\frac{\pi^2 E}{\sigma_p}} = \sqrt{\frac{\pi^2 \times 2 \times 10^5}{190}} = 100 \sqrt{\frac{\pi^2 \times 2}{19}} \approx 101$$



$$f_3 = \frac{Pb(3l^2 - 4b^2)}{48EI}$$

$$f_1 = \frac{P(3l^2 - 4b^2)}{48EI}$$

$$f_c = \frac{88P}{48EI}$$

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$$f_c = \frac{Pl}{48EI} = \frac{64N}{48EI}$$

$$\frac{N}{48EI} = \frac{88P}{48EI}$$

$$N = \frac{88P}{64} = \frac{11}{8}P = \frac{11}{8} \times 100 = 137.5 \text{ kN}$$

$$P_{cr} = \frac{\pi^2 EI}{(l)^2} = \frac{3.14^2 \times 2 \times 10^5 \times 2 \times 23.7 \times 10^4}{(0.7 \times 3)^2 \times 10^6} = 207.2 \text{ kN}$$

$$\frac{P_{cr}}{n_w} = 115.1 \text{ kN}$$

$$N > [P] = \dots$$

$$\begin{aligned} 9.1 \times 5 + 9.9 \times 5 &= 5 \times 5 \\ 9.1 \times 5 + 9.9 \times 5 &= 5 \times 5 \\ 9.1 \times 5 + 9.9 \times 5 &= 5 \times 5 \end{aligned}$$

七、用能量方法(单位力法或卡氏第二定理)求解图示超静定刚架反力, 刚架抗弯刚度 EI 已知, 不计剪切与轴向变形影响。(10分)

假设在 C 点施加单位力

$$\frac{\partial U}{\partial X_1} = \frac{\partial}{\partial X_1} \int_0^l (x_2 - py + x_1 y) dy$$

$$= \frac{\partial}{\partial X_1} \left(\frac{x_2 l^2}{2} - \frac{pl^3}{6} + \frac{x_1 l^3}{3} \right)$$

$$\frac{\partial U}{\partial X_2} = \frac{\partial}{\partial X_2} \left(\int_0^l x_2 dy + \int_0^l (x_2 - py) dy \right)$$

$$= \frac{\partial}{\partial X_2} \left(\frac{x_2 l}{2} + x_2 l - \frac{pl^2}{2} + \frac{x_1 l^2}{2} \right)$$

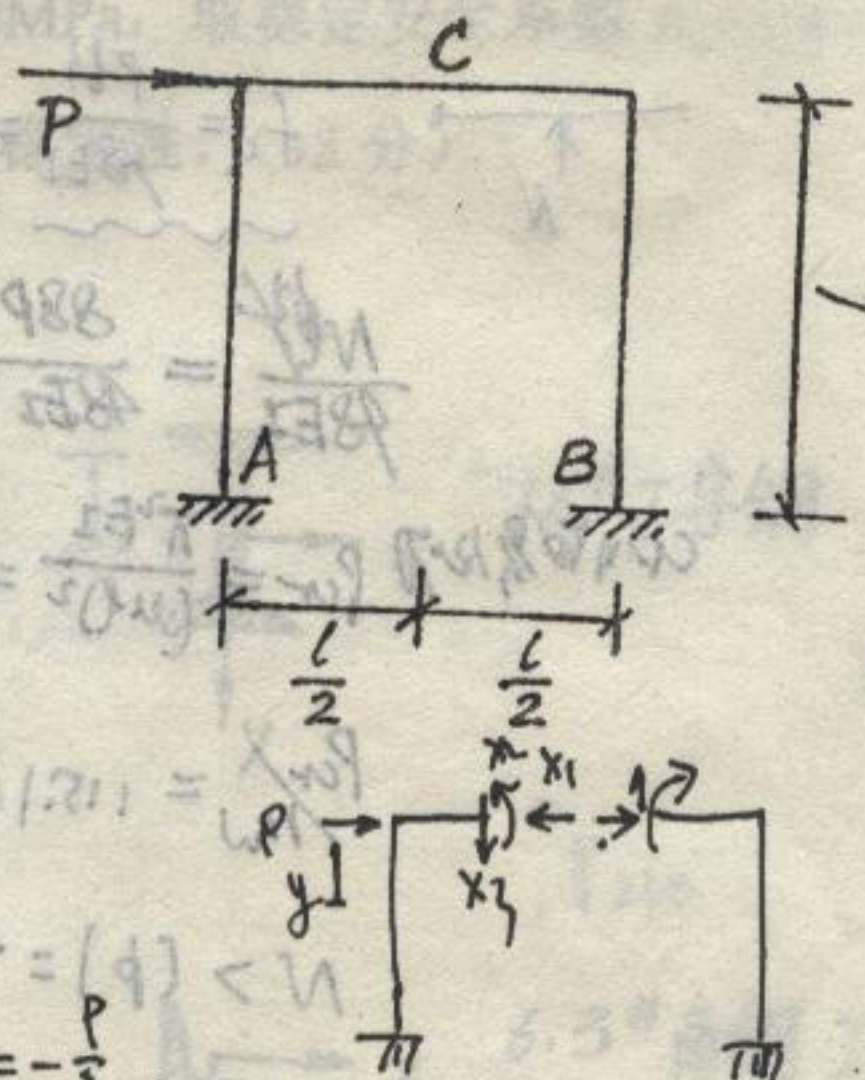
$$\frac{\partial U}{\partial X_1} = 0 \quad \frac{\partial U}{\partial X_2} = 0$$

$$\begin{cases} 3x_2 = 2pl + 2x_1 l \\ \frac{3x_2 l}{2} = \frac{pl^2}{2} - \frac{x_1 l^2}{2} \end{cases}$$

$$\begin{cases} 3x_2 = 2pl + 2x_1 l \\ 3x_2 = pl - x_1 l \end{cases}$$

$$\begin{cases} 3x_2 - 2pl + 2x_1 l = 0 \\ 3x_2 - pl + x_1 l = 0 \end{cases}$$

$$pl = x_1 l \quad x_1 = p$$



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